

**September 24, 2015  
 Invitation For Bid  
 Bridge Yard (IERBYS) Seismic and Renovation Project  
 Dated August 28, 2015 as Amended by Addendum No. 1  
 on September 11, 2015, Addendum No. 2 on September 18, 2015  
 and Addendum No. 3 on September 24, 2015**

Questions Received for Clarifications/Exceptions

**Questions & Answers #3**

Q1	Rivet Removal/Replacement Construction Sequence Note 3 requires abatement of the remaining paint under the rivet once the rivet is removed then this surface must be fayed for the pre-tensioned bolt per specs. If there is lead paint under the bolt this would necessitate a second mobilization of the abatement subcontractor. It is also likely that the lead paint was applied after the rivets were installed which would leave a relatively clean surface under the bolt. Please verify that there is paint underneath the rivets requiring abatement and please provide the lead content of that surface underneath a sample rivet.
A1	<i>It is highly likely, as stated, that the lead paint is exterior to the riveted connections. Since the rivet replacement operation is to be performed one at a time, the unexposed faces of the members -- which are likely to be paint-free -- will not be accessible. As outlined in S102, the Construction Sequence items 1, 2, and 3 on S102, provide the required procedure.</i>
Q2	Sheet S103 shows areas that are to receive negative pressure containment. Sheet S302 requires that knee truss framing is required at frames 2-17. This will require a significant amount of abatement outside negative pressure containment. Do the frames requiring strengthening per S303 require negative containment? If so, would it be permissible to provide negative pressure containment to the entire building.
A2	<i>S103 presents a suggested configuration for providing negative pressure enclosures for the smallest common repetitive units that require abatement. The Contractor may choose to provide negative containment to the entire building.</i>
Q3	Specification Section 99-088000 2.2 B1 Appearance does not include product numbers or mil thickness. Please provide specific products and mil thickness.
A3	<i>See Addendum #3.</i>
Q4	Specification Section 99-088000 2.2 B5 Acceptable Products lists Scotchcal & 3M products. Scotchcal manufactures the signage film and 3M manufactures W1 & W3. Will alternate manufactures be accepted as substitutes for these products?
A4	<i>See Addendum #3.</i>
Q5	What is to be the finish on the high strength bolts for both the rivet removal and structural strengthening of the frames? Are these bolts galvanized, zinc plated or carbon fasteners?

A5	<i>The A325 High Strength bolts need not be galvanized nor zinc coated. The structure will be painted after the retrofit is completed.</i>
Q6	Will TC bolts be acceptable for the high strength bolts for the structural strengthening of the frames?
A6	<i>TC (tension control) bolts will be acceptable, as indicated in the specifications (99-051205 Section 2.2B5.)</i>
Q7	Detail 7/S303 shows bolts being removed and replaced. Would removing and replacing one bolt at a time be sufficient to ensure this structure is secure during steel erection or will these members need to be temporary supported?
A7	<i>At the connection of the lower bay roof beam to the column, a minimum of two bolts must be removed at the same time for column flange interior plate installation (see Detail 7 on S322). This being the case, temporary support will be required.</i>
Q8	Detail 7/S303 shows bolts going through columns who have cladding preventing abatement of the outside face of the column flange. Negative pressure containment was required in other areas while abatement is occurring. What precautions need to be taken when abating the outside flanges of columns bounded by exterior finishes?
A8	<i>The negative pressure enclosures shown on S103 are suggested configurations that are applicable in the low bay as well.</i>
Q9	A4 speaks of shimming the reinforcing plates within the existing columns. Due to the age of the column members and the large amount of shop welding shown on the plate assemblies we are concerned that the warping of the new material and variances in the tolerances in the existing members will cause field fit up issues. Shimming these members would be the best way to mitigate fit-up risk. Up to a ¼” on each side or ½” total would help to reduce the risk. Will shimming to this extent be allowed? Also, will the shims need to be continuous or could they be simple shims that can be inserted between the bolts? Please advise.
A9	<i>As stated, shimming for installation of column web reinforcement will be permitted. It is expected that the total variance shall be controlled in such a way as to not exceed 3/16” total (combined for both sides). The shims shall be continuous along the length of the plate.</i>
Q10	The welded splice shown on S321 shows a full penetration weld being performed to splice field plates together. When these plates are in place it will be impossible to perform a backup bar required for this type of weld? Will the engineer approve using the existing column as a backup bar? Or alternatively allow a partial joint penetration in lieu of the full penetration weld?
A10	<i>The detail for C.J.P. for splicing plates in the field is shown as optional, and as indicated in Note 5 the welding shall be performed before lifting the plates up for installation. Using the existing column as a backup bar is not allowed.</i>
Q11	The revised standard specs contain a document entitled Required contract provisions Federal-Aid Construction Contracts FHWA-1273 Revised May 1, 2012. Section VII

	entitled Subletting or Assigning The Contract subsection 1 states that there is a 30% self-perform requirement on the project. Due to the large amount of structural retrofit, abatement and sole sourced products on the project it will not be possible for many of the general contractors looking to bid this project to meet such a requirement. For this reason, we ask that this requirement be removed by addenda.
A11	<i>No adjustment will be made to the self-perform requirement on the project.</i>
Q12	A301 Keynote 51 a) states that cracks, voids and holes in transite panels shall be repaired with a repair patch by 3M companies. Please provide what specific product shall be used and a specification for application.
A12	<i>See Addendum #3.</i>
Q13	A301 Keynote 51 a) states that cracks, voids and holes in transite panels shall be repaired with a repair patch by 3M companies. It is not possible to obtain an accurate access how much cracking is present on site from the contract documents. For this reason, we request that this work be broken out as a separate bid item with a fixed quantity provided on a new bid form issued via addenda.
A13	<i>See Addendum #3.</i>
Q14	The note states that all broken glazing lites be replaced. Please provide a specification and procedure on how this it to be performed.
A14	<i>Refer to Specifications Section 99-088000, Part 3 Execution, for related information. It is up to the Contractor to determine how to perform the replacement of broken glazing lites.</i>
Q15	The note states that all broken glazing lites be replaced. It is not possible to obtain an accurate quantity on the number of lites that need to be replaced. For this reason, we request that this work be broken out as a separate bid item with a fixed quantity provided on a new bid form issued via addenda.
A15	<i>Refer to Specifications Section 99-088000, Part 1.2, B.3.</i>
Q16	Please confirm that Standard Specification 55 Steel Structures is applicable to this project.
A16	<i>Yes, it is applicable, as modified per the Special Provisions.</i>
Q17	Please confirm that the welding of steel is subject to AWS D1.5. If so please identify what members are fracture critical.
A17	<i>As provided in the contract documents, the welding shall meet the requirements of AWS D1.1 and the seismic supplement AWS D1.8. The fracture critical members for this structure are the bottom chords and the tension diagonals of the transverse trusses, where the retrofit provisions involve drilling holes for bolt installation – welding is not specified for these members.</i>
Q18	Specifications 55-1.01C(2) states review times of 45 to 60 days for the various structures listed. What will be the review time for the shop drawings on this project?

A18	<i>The shop drawing review period shall be less than 20 working days.</i>
Q19	Will bolting be subject to the requirements of 55 Steel Structures in the shop and field?
A19	<i>All bolting (procurement, testing, QA/QC, etc.) shall comply with Section 55 of the Standard Specifications as modified by the Special Provisions.</i>
Q20	Will thermal cutting of holes be permitted if it complies with AISC standards?
A20	<i>Thermal cutting of holes in existing structural members (columns, beams, truss members, etc.) will not be permitted. However, it will be permitted in the fabrication of new plates in the shop, provided it meets all the requirements of AISC.</i>
Q21	We have had requests from several subcontractors to view the interior of the building. Will another non-mandatory walk be scheduled or is there another way to schedule access to the building?
A21	<i>A second project site visit is scheduled for Monday, September 28, 2015 at 10:30 am. Bidders interested in attending this site visit should arrive on time and meet at 210 Burma Rd., Oakland, CA 94607.</i>
Q22	Reference is made to details shown on 3/A6.01. Please confirm how new 1-1/4 x 1-1/4 steel angle is attached to the supporting (E) steel angle or (E) steel track.
A22	<i>New steel angle is attached to existing steel angle using self-tapping sheet metal screws, 9 inches on center.</i>
Q23	Please confirm requirements for reputtying of existing undamaged glazing. Can the existing glazing remain in place after existing putty is removed and new putty be installed without removing glazing?
A23	<i>The intent is for undamaged original glazing lites to remain. It is up to the Contractor to come up with means and methods for how to reputty existing undamaged glazing.</i>